

Materials, Construction, and Emerging Best Practices for Exterior Slab Concrete

Spring 2026 National Concrete Consortium

San Diego, CA

April 8, 2026



Presenters

Richard Mulcahy, P.E.

Materials Research and Evaluation Engineer

MassDOT Research and Materials Section

Email: Richard.Mulcahy@dot.state.ma.us

Oussama Khouchani, Ph.D.

Research and Evaluation Technical Lead

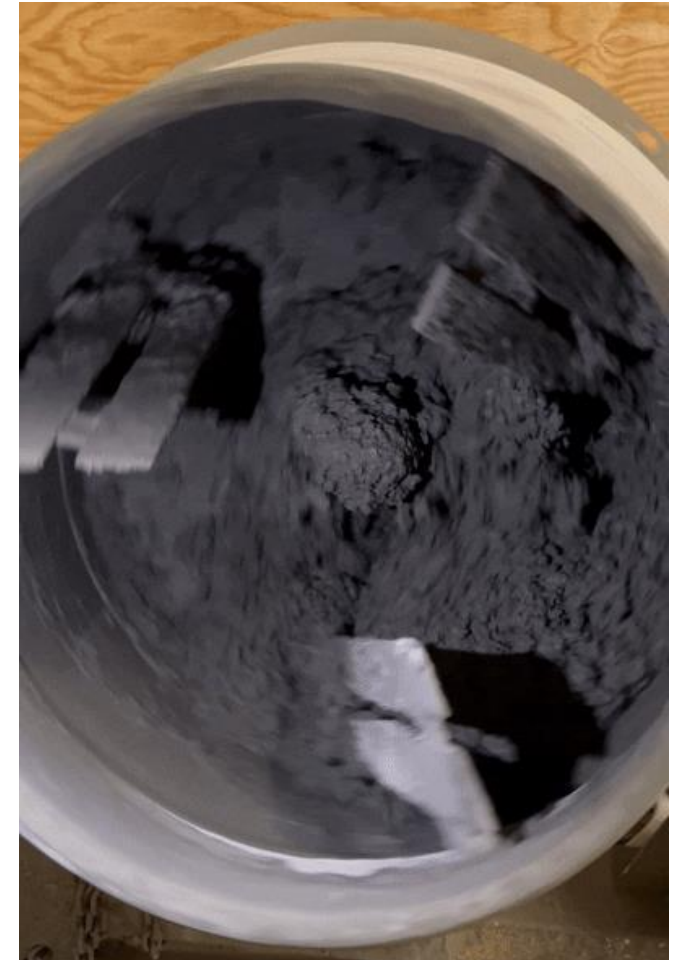
MassDOT Research and Materials Section

Email: Oussama.A.Khouchani@dot.state.ma.us



Learning Outcomes

1. **Scaling Deterioration Mechanisms and Causes**
2. **Materials and Construction Best Practices**
3. **Field Investigations and Petrographic Examination**
4. **Emerging Chemical Admixtures and Topical Treatments**





Scaling Deterioration

Understanding the mechanisms and causes.



Spring 2026 – National Concrete Consortium

Definition

“A general loss of surface mortar or mortar surrounding the coarse aggregate particles on a concrete”

– American Cement Association

“Local flaking or peeling away of the near-surface portion of hardened concrete”

– American Concrete Institute

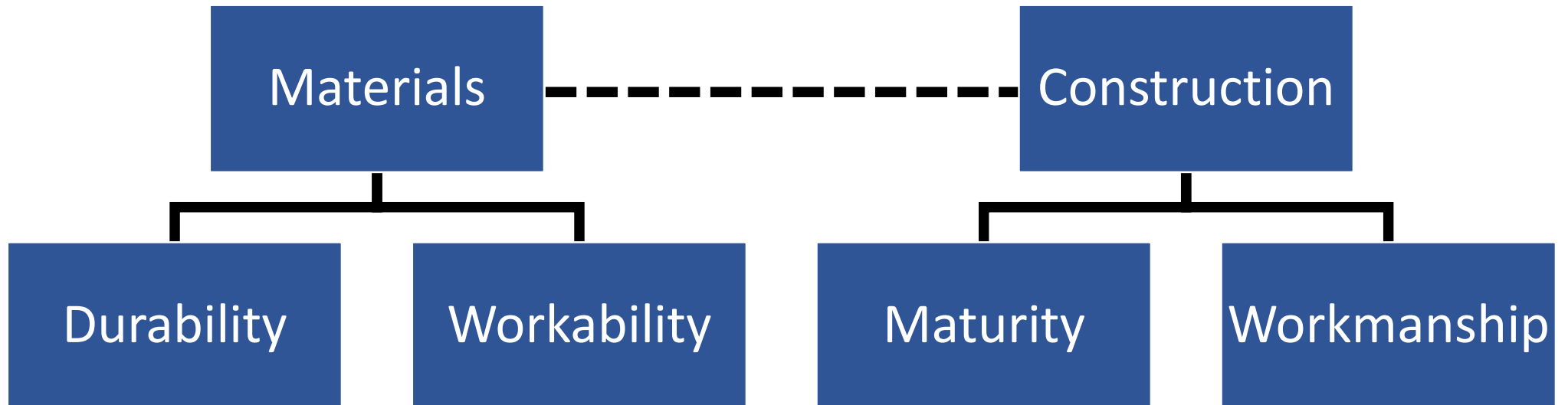


Causes



Causes

Scaling is Susceptible to concrete with insufficient...



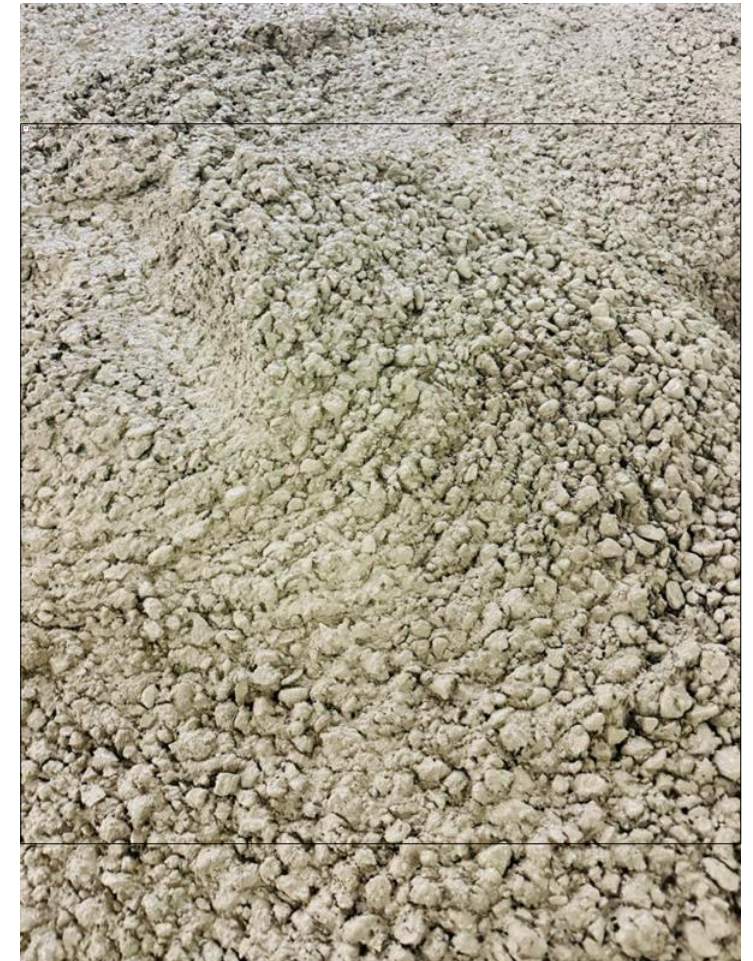
Causes – Materials

Insufficient Durability

- Poor air entrainment
- High water to cementitious ratio
- Unaccounted for free moisture on aggregate
- High permeability

Insufficient Workability

- Poor air, combined aggregate, and paste systems
- Improper dosage or insufficient usage of chemical admixtures
- Unaccounted for absorption of aggregate
- Too much water “held back” at the plant
- Improper “re-tempering” of hold-back water
- Deviation from the approved mix design



Causes – Construction

Insufficient Maturity

- Incomplete / interrupted hydration due to exposure to adverse conditions prior to the required protection period is complete

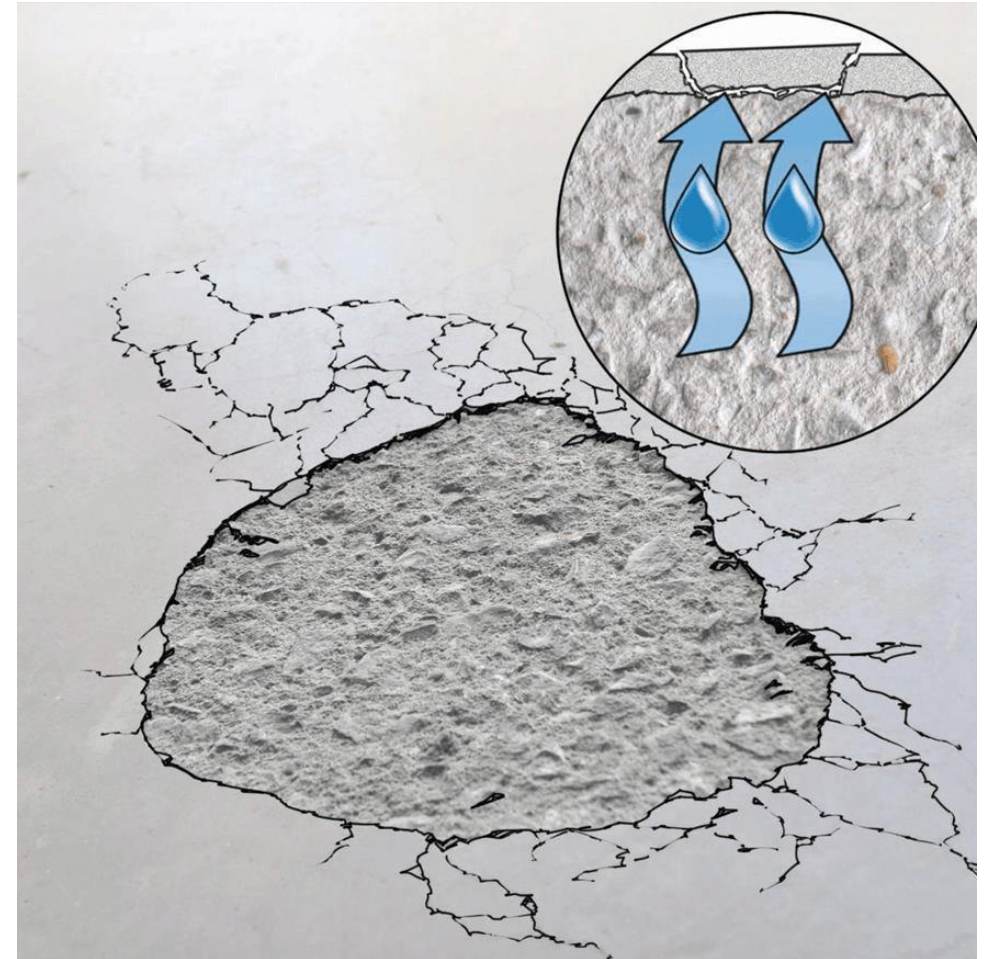
Improper Workmanship

- Lack of personnel certification for concrete flatwork and insufficiently sized work crew
- Unfamiliarity with the initial and final time of set for a given exterior slab concrete mix design
- Improper finishing methods
- Lack of protection from adverse conditions (precipitation, cold conditions, and hot conditions)
- Inadequate curing methods
- Use of prohibited tools and conducting prohibited practices

Mechanisms

Delamination

- Trapped bleed water (and air) under the hardened paste from use of prohibited finishing tools (steel trowels) or final finishing operations conducted too early (bleed water still rising)
- Bleed water (and air) collects under the hardened paste layer resulting in no bond with the substrate
- Insufficient application of evaporation reducing materials, causing moisture loss and surface “crusting” while the concrete is still plastic beneath
- Hardened paste layer chips off



Mechanisms

Weak Top Paste Layer

- “Blessing” the surface
- Finishing in bleed water
- Finishing in evaporation reducers
- Insufficient curing
- Insufficient protections from adverse conditions
- Results in substantially weak, porous, and carbonated top paste layer



PROPER APPLICATION

APPLY DURING EVAPORATIVE CONDITIONS TO PREVENT RAPID MOISTURE LOSS.

WARNING

NOT A FINISHING AID.
DO NOT FINISH INTO OR UNDER THE SURFACE.

Mechanisms

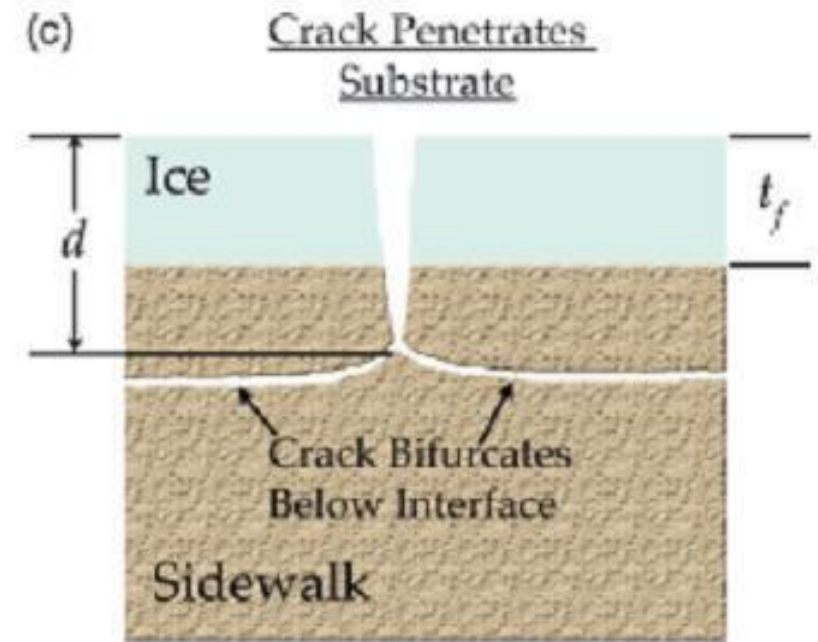
Weak Top Paste Layer – Internal Pressures

- Water in the concrete pore solution solidifies into ice
- As the temperature drops below freezing, the pore water freezes and increases 9% in total volume, causing internal pressures inside the concrete
- Dissolved chlorides in the pore solution are also expelled from the solution as it freezes, causing even more internal pressures
- Weak top layer scales

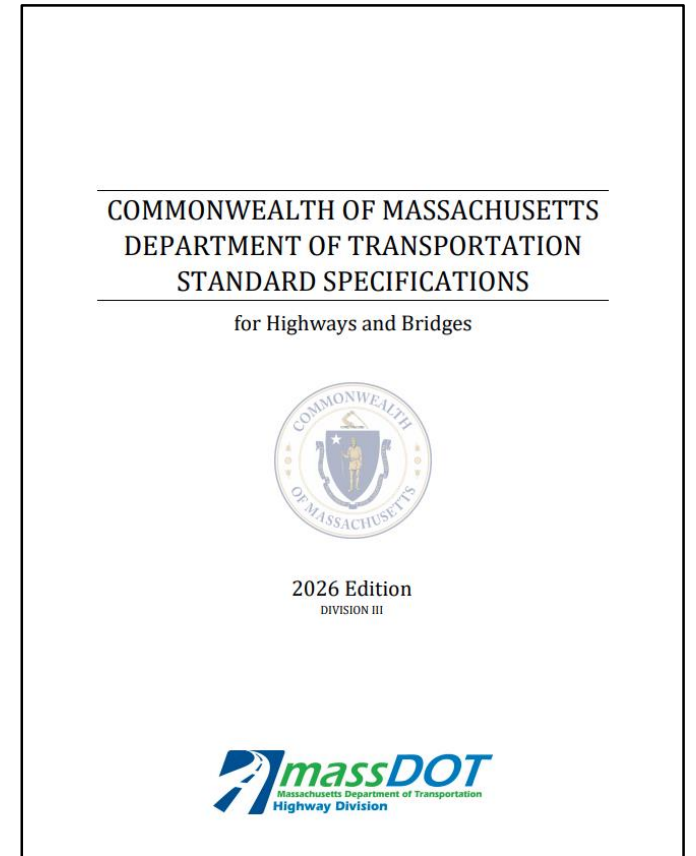
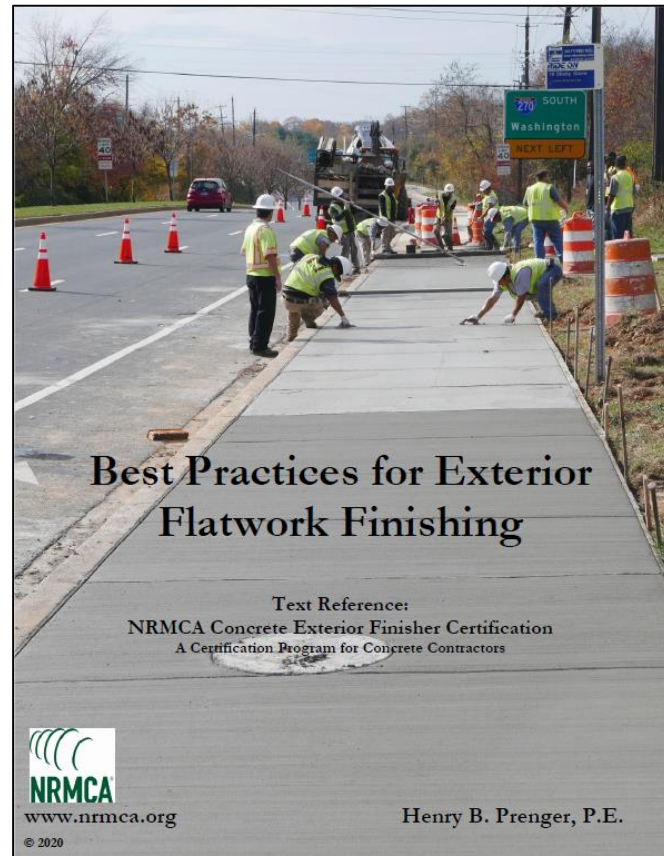
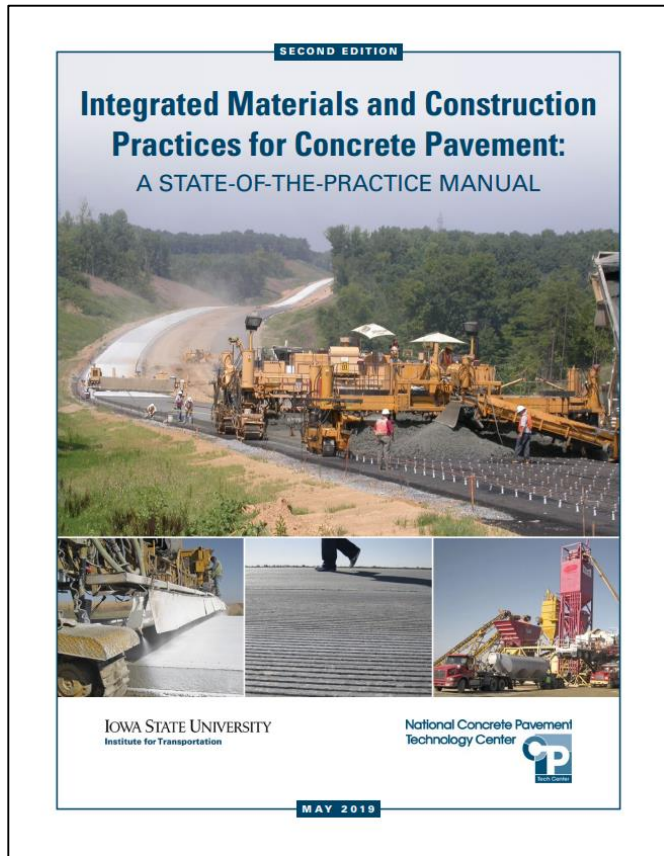
Mechanisms

Weak Top Paste Layer – Glue Spall Mechanism

- Water (or brine solution) freezes onto concrete surface creating a composite
- Composite temperature decreases, stresses form from thermal expansion mismatch between ice and concrete
- Ice thermal expansion coefficient five times higher than concrete
- Ice is put in tension and fractures
- Cracks in ice penetrate the concrete substrate up to a depth (d) of 0.75 times thickness of the ice layer (t_f)
- Weak top layer scales



Valenza and Scherer (2006)



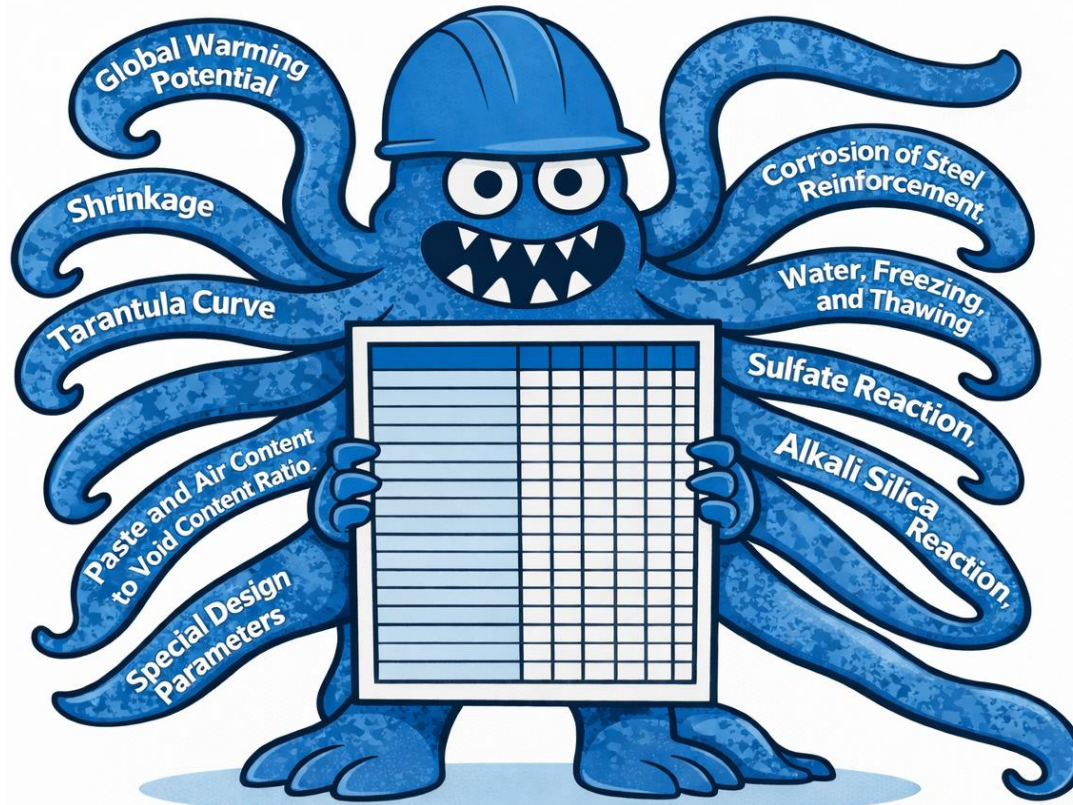
Materials and Construction Best Practices

Getting back to the basics...



Spring 2026 – National Concrete Consortium

Mix Design Formulation



MONSTER CHART



Materials and Construction Best Practices

Mix Design Formulation

Mix Design Parameters	C C	H P C	H E S C	R H C	L W C	S C C	E S C	P C	M P C	F R C	U H P C	S H O T	U W C	D C	L M C	I C C
Alkali Silica Reaction	X	X	X	X	X	X	X	X	X	X	-	X	X	X	X	X
DEF and Thermal Cracking	-	-	-	-	-	-	-	-	X	-	-	-	-	-	-	-
Corrosion of Steel Reinforcement	X	X	X	X	X	X	-	-	X	X	-	-	X	-	-	X
Water, Freezing, and Thawing	X	X	X	X	X	X	X	X	X	X	-	X	X	-	-	X
Sulfate Reaction	X	X	X	X	X	X	X	X	X	X	-	X	X	-	-	X
Shrinkage	-	X	X	-	X	X	X	X	X	-	-	-	-	-	-	-
Global Warming Potential	When Specified										-	-	-	-	-	-
Shilstone Workability-Coarseness	-	X	X	-	-	-	X	X	X	-	-	-	-	-	-	-
Tarantula Curve	-	-	-	-	-	-	X	X	-	-	-	-	-	-	-	-
Paste and Air Content to Void Content Ratio	-	X	X	-	-	-	X	X	X	-	-	-	-	-	-	-
Special Design Parameters	-	X	X	X	X	X	-	-	-	X	X	X	X	-	X	X

Mix Design Formulation

Effects on Concrete Properties from Incorporation of SCMs [1]

Property		Coal Ash (F)	Coal Ash (C)	Calcined Shale	Calcined Clay	Meta-kaolin	Slag	Silica Fume
Fresh	Water Content	↓	↓	↔	↔	↑	↓	↑
	Workability	↑	↑	↑	↑	↓	↑	↓
	Bleeding and Segregation	↓	↓	↔	↔	↓	↕	↓
	Setting Time	↑	↕	↔	↔	↔	↑	↔
	Air Content	↓	↓	↔	↔	↓	↔	↓
	Heat of Hydration	↓	↕	↓	↓	↔	↓	↕

[1] ↑ = Increased; ↓ = Decreased; ↕ = Increased or Decreased; ↔ = No Impact

Mix Design Formulation

Effects on Concrete Properties from Incorporation of SCMs [1]

Property		Coal Ash (F)	Coal Ash (C)	Calcined Shale	Calcined Clay	Meta-kaolin	Slag	Silica Fume
Hardened	Curing Duration	↑	↑	↑	↑	↑	↑	↑
	Early Age Strength	↓	↔	↓	↓	↑	↕	↑
	Long Term Strength	↑	↑	↑	↑	↑	↑	↑
	Abrasion Resistance	↔	↔	↔	↔	↔	↔	↔
	Drying Shrinkage and Creep	↔	↔	↔	↔	↔	↔	↔
	Permeability	↓	↓	↓	↓	↓	↓	↓
	Corrosion	↓	↓	↓	↓	↓	↓	↓
	Alkali Silica Reaction	↓	↓	↓	↓	↓	↓	↓
	Sulfate Reaction	↓	↕	↓	↓	↓	↓	↓
	Freezing and Thawing	↔	↔	↔	↔	↔	↔	↔
Scaling	↔	↔	↔	↔	↔	↔	↔	

[1] ↑ = Increased; ↓ = Decreased; ↕ = Increased or Decreased; ↔ = No Impact

Materials and Construction Best Practices: Mix Design Formulation



2026 CEMENT CONCRETE MIX DESIGN SHEET

RMS 043

PLANT INFORMATION			MAILING ADDRESS			MIX DESIGN SHEET IDENTIFICATION		
PLANT NAME	LOCATION	STREET NO. & ADDRESS	CITY/TOWN	EMAIL ADDRESS	CONTRACT	SHEET IDENTIFICATION NO.		
BORO SAND AND STONE	EASTON, MA	192 Plain Street	North Attleboro, MA 02760	dougs@borocorp.com		25-03-26-10-27-01		

CONSTITUENT MATERIALS																								
ID	SOURCE	LOCATION	NMA S	DESCRIPTION	SPEC.	SG	UW _{DR} (PCF)	VC (%)	PERCENT PASSING BY MASS (%)												FM			
									2 IN.	1 1/2 IN.	1 IN.	3/4 IN.	1/2 IN.	3/8 IN.	#4	#8	#16	#30	#50	#100		#200		
FINE	BORO (80%) / RYCO (20%)	N. ATTLEBORO / CARVER, MA	FINE	NORMAL WEIGHT	M 6	2.63	108.3	33.9	100.0	100.0	100.0	100.0	100.0	100.0	100.0	97.0	85.0	68.0	49.0	25.0	8.0	2.0	2.68	
CA1	CUMBERLAND QUARRY	CUMBERLAND, RI	3/8 IN.	NORMAL WEIGHT - 8	M 80	2.71	103.9	38.4	100.0	100.0	100.0	100.0	100.0	100.0	98.0	30.0	9.0	4.6	0.0	0.0	0.0	0.0	0.0	5.58
CA2	CUMBERLAND QUARRY	CUMBERLAND, RI	3/4 IN.	NORMAL WEIGHT - 57	M 80	2.70	99.1	41.1	100.0	100.0	100.0	99.0	58.0	13.0	2.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	6.86
CA3	CUMBERLAND QUARRY	CUMBERLAND, RI	1 1/2 IN.	NORMAL WEIGHT - 4	M 80	2.81	94.6	45.9	100.0	99.0	43.0	10.0	0.0	1.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	7.90

CEMENT; SUPPLEMENTARY CEMENTITIOUS MATERIALS; PACKAGED; FIBERS										CHEMICAL ADMIXTURES						V _s (%)
ID	SOURCE	LOCATION / PRODUCT	TYPE	DESCRIPTION	SPEC.	SG	ID	SOURCE	PRODUCT	TYPE	DESCRIPTION	SPEC.				
CEM	ST MARYS CEMENT	GASCONS, QC (IL(10)MS)	L(10)MS	10% LIMESTONE BLEND (MS)	M 240	3.11	AD1	CHRYSO INC	DAREX II AEA	AEA	AIR ENTRAINING	M 154	9.90			
SCM1	AMRIZE (HOLCIM)	BALTIMORE, MD	S-120	HIGH ACTIVITY (120)	M 302	2.88	AD2	CHRYSO INC	RECOVER	D	WATER REDUCING AND RETARDING	M 194	15.53			
SCM2							AD3	CHRYSO INC	ADVA 140M	F	HIGH RANGE WATER REDUCING	M 194	12.66			
SCM3							AD4	CHRYSO INC	ADVA 190	F	HIGH RANGE WATER REDUCING	M 194	26.73			
PKG							AD5	CHRYSO INC	DCI-S	CIA	CORROSION INHIBITING	C1582	22.00			
FIBER							AD6	SIKA	LIGHTCRETE POWDER	CLSM	CLSM ENHANCING	TDS	100.00			

Type II and Slag

MIX DESIGN FORMULATION																														
MIX DESIGN ID NO. DOT	PRODUCER	MIX DESIGN TYPE	C (PSI)	NMA S (IN.)	S (IN.)	AC (%)	W/CM RATIO	PC (%)	AGGREGATE (LBS.)				CEMENTITIOUS; PACKAGED; FIBER (LBS.)					WATER (GAL.)		ADMIXTURES (FL. OZ.)						YIELD (CF)	UW (PCF)			
									CA1	CA2	CA3	CEM	SCM1	SCM2	SCM3	PKG	FIBER	W _f	W _{ADMIX}	AD1	AD2	AD3	AD4	AD5	AD6					
01	DPWS4034	CC	4000	3/4	5.00	6.0	0.44	30.4	1109.0	525.0	1225.0	329.0	329.0						35.0	0.2	2.6								27.00	141.1
02	DPWS40112	CC	4000	1 1/2	5.00	6.0	0.42	29.4	1029.0	304.0	988.0	608.0	329.0	329.0					33.0	0.3	2.0			39.0					27.00	143.1
03	DPWHP5034	HPC ESC	5000	3/4	6.00	7.0	0.39	29.8	1089.0	465.0	1275.0		343.0	295.0	47.0				32.0	3.0	3.2			19.0	26.0	384.0			27.00	140.4
04	FEGG	CLSM MANUFACTURED	4000	FINE	10.00	25.0	0.34	25.2	2202.0				75.0						48.0	0.0									27.00	89.2
05	DPWS4034SW	ESC	4000	3/4	5.00	7.0	0.44	28.5	1178.0	450.0	1270.0		310.0	310.0					32.7	0.3	3.0			36.6					27.00	140.4
07													300.0						30.0	0.0								0.5	27.00	112.8
08													150.0						35.0	0.0								0.8	27.00	101.2
09													292.0	258.0	35.0				28.1	3.0	3.0			17.5	23.4	384.0			27.00	142.1
10													2015.0						114.0	0.4			66.0						27.00	109.9

Exterior Slab Concrete (ESC)

COMBINED AGGREGATE AND PASTE SYSTEM																												
MIX DESIGN ID NO. DOT	PRODUCER	PERCENT BY MASS PASSING (%)												ANDREA SEN G	TARANTULA CURVE	SHILSTONE ZONE	VOID CONTENT (%)			PC _a / VC _m	SCM (%)			TCM (LBS.)	ECC (LBS.)	THICK (FT.)		
		2 IN.	1 1/2 IN.	1 IN.	3/4 IN.	1/2 IN.	3/8 IN.	#4	#8	#16	#30	#50	#100				#200	VC _a	VC _{T15}		VC _m	SCM1	SCM2				SCM3	
01	DPWS4034	100.0	100.0	100.0	99.6	82.0	62.4	44.0	34.6	27.2	19.0	9.7	3.1	0.8	0.46	82.8	OUTSIDE	II: 3/4 - 2 IN.	27.2		17.3	2.10	50.0	0.0	0.0	658.0	625.1	2.0
02	DPWS40112	100.0	99.8	88.2	81.0	65.1	49.9	37.9	30.8	24.4	17.2	8.8	2.8	0.7	0.35	84.4	OUTSIDE	II: 3/4 - 2 IN.	27.3		17.6	2.01	50.0	0.0	0.0	658.0	625.1	2.0
03	DPWHP5034	100.0	100.0	100.0	99.5	81.1	60.5	43.2	34.2	26.9	18.9	9.6	3.1	0.8	0.48	81.0	OUTSIDE	II: 3/4 - 2 IN.	27.5		17.3	2.13	43.1	6.9	0.0	685.0	694.4	1.5
04	FEGG	100.0	100.0	100.0	100.0	100.0	100.0	97.0	85.0	68.0	49.0	25.0	8.0	2.0	0.20	76.5	OUTSIDE	IV: TOO FINE	33.9		16.9	2.97	0.0	0.0	75.0	75.0	10.0	
05	DPWS4034SW	100.0	100.0	100.0	99.6	81.6	61.6	45.0	35.9	28.4	19.9	10.2	3.3	0.8	0.45	81.0	OUTSIDE	II: 3/4 - 2 IN.	27.4		17.7	2.01	50.0	0.0	0.0	620.0	589.0	2.5
06	DPWS4038	100.0	100.0	100.0	100.0	100.0	98.9	59.8	42.8	32.8	21.8	11.1	3.6	0.9	0.51	85.9	OUTSIDE	III: < 3/4 IN.	26.6		16.0	2.49	50.0	0.0	0.0	705.0	669.8	1.5
07	FEGGNEXC	100.0	100.0	100.0	100.0	100.0	100.0	97.0	85.0	68.0	49.0	25.0	8.0	2.0	0.20	76.5	OUTSIDE	IV: TOO FINE	33.9		19.1	2.28	0.0	0.0	300.0	300.0	6.0	
08	FEGGMEXC	100.0	100.0	100.0	100.0	100.0	100.0	97.0	85.0	68.0	49.0	25.0	8.0	2.0	0.20	76.5	OUTSIDE	IV: TOO FINE	33.9		17.6	2.74	0.0	0.0	150.0	150.0	10.0	
09	DPWHP4034	100.0	100.0	100.0	99.6	82.2	62.8	46.3	37.2	29.3	20.6	10.5	3.4	0.8	0.42	81.5	OUTSIDE	II: 3/4 - 2 IN.	27.4		18.3	1.79	44.1	6.0	0.0	585.0	592.0	2.5
10	50NEAT														#####	#####	OUTSIDE		#####		#####		0.0	0.0	0.0	2015.0	2015.0	

We agree to produce cement concrete mix designs per the precise proportions, quantities, types, and sources of constituent materials identified on the approved RMS 043 Cement Concrete Mix Design Sheet for MassDOT construction contracts.

Mr. Doug Smith
NAME

Quality Control
TITLE

AUTHORIZED SIGNATURE

4/7/2026
DATE

Materials and Construction Best Practices Mix Design Formulation

MIX DESIGN FORMULATION

MIX DESIGN ID NO.		C (PSI)	NMAS (IN.)	S (IN.)	AC (%)	W/CM RATIO	PC (%)	AGGREGATE (LBS.)			CEMENTITIOUS; PACKAGED; FIBER (LBS.)					WATER (GAL.)		ADMIXTURES (FL. OZ.)						YIELD (CF)	UW (PCF)			
DOT	PRODUCER							MIX DESIGN TYPE	FINE	CA1	CA2	CA3	CEM	SCM1	SCM2	SCM3	PKG	FIBER	W _T	W _{ADMIX}	AD1	AD2	AD3			AD4	AD5	AD6
05	DPWS4034SW	ESC	4000	3/4	5	7.0	0.44	28.5	1178.0	450.0	1270.0	0.0	310.0	310.0	0.0	0.0	0.0	0.0	32.7	0.3	3.0	0.0	36.6	0.0	0.0	0.0	27.00	140.4

OPTIMIZED COMBINED AGGREGATE AND PASTE SYSTEM ANALYSIS

PERCENT BY MASS PASSING (%)													PERCENT RETAINED BY MASS (%) ON EACH SIEVE					SHILSTONE WORKABILITY-COARSENESS FACTOR			TCM (LBS.)	ECC (LBS.)	THICK (FT.)																																														
2 IN.	1 1/2 IN.	1 IN.	3/4 IN.	1/2 IN.	3/8 IN.	#4	#8	#16	#30	#50	#100	#200	TARANTULA CURVE RESULT					CF	WF	ZONE	(LBS.)	(LBS.)	(FT.)																																														
100.0	100.0	100.0	99.6	81.6	61.6	45.0	35.9	28.4	19.9	10.2	3.3	0.8	CLOSE CONFORMITY					60.0	37.4	II: 3/4 - 2 IN.	620.0	589.0	2.5																																														
Modified Andraesen Distribution <table border="1"> <thead> <tr> <th>FM</th> <th>CA (%)</th> <th>q</th> <th>E (%)</th> <th>VC_A (%)</th> </tr> </thead> <tbody> <tr> <td>4.96</td> <td>55.0</td> <td>0.45</td> <td>81.0</td> <td>27.4</td> </tr> </tbody> </table>													FM	CA (%)	q	E (%)	VC _A (%)	4.96	55.0	0.45	81.0	27.4	Paste Content (%) <p>PC (%) 28.5</p>		PC_A / VC_M Ratio <table border="1"> <thead> <tr> <th>PC_A (%)</th> <th>VC_M (%)</th> <th>RATIO</th> </tr> </thead> <tbody> <tr> <td>35.5</td> <td>17.7</td> <td>2.01</td> </tr> </tbody> </table>		PC _A (%)	VC _M (%)	RATIO	35.5	17.7	2.01	Tarantula Curve <p>OPTIMIZED GRADATION</p>					Shilstone Chart <p>ZONE II (OPTIMUM FOR NMAS 3/4 IN. - 2 IN.): OPTIMIZED WORKABILITY FACTOR AND COARSENESS FACTOR</p>			DEF/Thermal Crack Resistance <table border="1"> <thead> <tr> <th>T_{MAX} (°F)</th> <th>T_C (°F)</th> <th>T_A (°F)</th> </tr> </thead> <tbody> <tr> <td>158.00</td> <td>70</td> <td>60</td> </tr> </tbody> </table> <table border="1"> <thead> <tr> <th>SPECIFIC APPLICATION</th> <th>THICK. (FT.)</th> </tr> </thead> <tbody> <tr> <td>DS (SOIL)</td> <td>9.00</td> </tr> <tr> <td>DS (CASING)</td> <td>9.00</td> </tr> <tr> <td>DS (WATER)</td> <td>9.00</td> </tr> <tr> <td>ABUTMENT</td> <td>1.22</td> </tr> <tr> <td>PIER (ABOVE)</td> <td>1.52</td> </tr> <tr> <td>PIER CAP</td> <td>1.80</td> </tr> <tr> <td>PILE CAP</td> <td>2.68</td> </tr> <tr> <td>BRIDGE DECK</td> <td>9.00</td> </tr> <tr> <td>SLAB (GRADE)</td> <td>9.00</td> </tr> </tbody> </table>			T _{MAX} (°F)	T _C (°F)	T _A (°F)	158.00	70	60	SPECIFIC APPLICATION	THICK. (FT.)	DS (SOIL)	9.00	DS (CASING)	9.00	DS (WATER)	9.00	ABUTMENT	1.22	PIER (ABOVE)	1.52	PIER CAP	1.80	PILE CAP	2.68	BRIDGE DECK	9.00	SLAB (GRADE)	9.00
FM	CA (%)	q	E (%)	VC _A (%)																																																																	
4.96	55.0	0.45	81.0	27.4																																																																	
PC _A (%)	VC _M (%)	RATIO																																																																			
35.5	17.7	2.01																																																																			
T _{MAX} (°F)	T _C (°F)	T _A (°F)																																																																			
158.00	70	60																																																																			
SPECIFIC APPLICATION	THICK. (FT.)																																																																				
DS (SOIL)	9.00																																																																				
DS (CASING)	9.00																																																																				
DS (WATER)	9.00																																																																				
ABUTMENT	1.22																																																																				
PIER (ABOVE)	1.52																																																																				
PIER CAP	1.80																																																																				
PILE CAP	2.68																																																																				
BRIDGE DECK	9.00																																																																				
SLAB (GRADE)	9.00																																																																				

PC (%) > 28.0
PC_A / VC_M > 1.75

No segregation and good workability observed during the trial batch.

Town/City: STATEV
 Contractor: #N/A
 Report to District: RMS
 Resident Engineer: #N/A

Bid Item: VER.M4
 Bid Item Description: VERIFIC
 Sub-Item Description: VER.M4
 Bid Item Quantity: #N/A
 MassDOT Mix ID No.: 25-03-26
 Produced by: BORO S
 Design Strength (psi): 4000
 Tot. Cementitious (lbs.): 620
 Proposed Use: CONCR

Date Sampled: 5/2/2025
 Sampling Location: BORO S
 Truck No.: 181
 Sample Time: 8:05 AM
 Job Water Added: NO
 Random Sample: Y
 Quantity Represented: Y

PREPA
 Specimen Size: 4 x 8"
 Specimens Covered: Yes

Test Method	Quality Chara
T 119 (C143)	Slump (f
T 152 (C231)	Air Conten
T 309 (C1064)	Concrete Ter
T 121 (C231)	Unit Weigh

FIELD
 Office: RMS
 Tested By: MIKE NAUGHT
 Witnessed By: ROBERT BOU
 Reviewed By: Colin O'Brien

Results are within specification li
 by typing my name below, I understand
 Approved By: Colin O'Brien

Town/City: STATEWIDE
 Contractor: #N/A
 Report to District: RMS
 Resident Engineer: #N/A

CHLORIDE ION PENETRATION RESISTANCE TESTING (T 358)

Specimen	Cylinder 1		Cylinder 2		Cylinder 3		Set Average Resistivity Determination (kΩ-cm)
	1st Test	Repeat	1st Test	Repeat	1st Test	Repeat	
00086139	73.5		73.6		73.5		Age (Days): 7 Curing Factor: 1.0 Cyl. 1 Average: 16.6 Cyl. 2 Average: 17.2 Cyl. 3 Average: 17.0 Set Average: 17 Penetrability: Moderate Spec. Min.: - Pass/Fail: FIO
Temp. (°F)	73.5		73.6		73.5		
Angle	1st Test	Repeat	1st Test	Repeat	1st Test	Repeat	
0°	16.1		17.5		17.5		
90°	16.5		17		17.1		
180°	17.1		16.9		16.5		
270°	16.9		17.5		16.7		
0°	16		17.6		17.5		
90°	16.5		17.1		17.2		
180°	17.1		16.8		16.5		
270°	16.9		17.5		16.8		
Average	16.6	Not Req'd	17.2	Not Req'd	17.0	Not Req'd	
%CV	2.6%	Not Req'd	1.9%	Not Req'd	2.4%	Not Req'd	
Average 16	Not Req'd		Not Req'd		Not Req'd		

Specimen	Cylinder 1		Cylinder 2		Cylinder 3		Set Average Resistivity Determination (kΩ-cm)
	1st Test	Repeat	1st Test	Repeat	1st Test	Repeat	
00086142	75.6		75.5		75.3		Age (Days): 28 Curing Factor: 1.0 Cyl. 1 Average: 34.1 Cyl. 2 Average: 36.5 Cyl. 3 Average: 37.5 Set Average: 36 Penetrability: Low Spec. Min.: - Pass/Fail: FIO
Temp. (°F)	75.6		75.5		75.3		
Angle	1st Test	Repeat	1st Test	Repeat	1st Test	Repeat	
0°	33.6		37.4		36.5		
90°	31.4		36.2		37.1		
180°	35.1		35.2		39.7		
270°	36.5		36.8		36.7		
0°	33.5		37.5		36.5		
90°	31.5		36.4		37		
180°	34.9		35.3		39.8		
270°	36.6		36.8		36.8		
Average	34.1	Not Req'd	36.5	Not Req'd	37.5	Not Req'd	
%CV	5.9%	Not Req'd	2.4%	Not Req'd	3.7%	Not Req'd	
Average 16	Not Req'd		Not Req'd		Not Req'd		

Calculate the average and the %CV for each sample in the set. If the %CV is > 7.5%, immerse sample in water bath (68 to 77 °F) for 2 h, and record results in the "Repeat" Column. If the %CV on the "Repeat" Set is < 7.5%, use the average of the "Repeat" Set. If the %CV is > 7.5%, average all 16 readings.

Town/City: STATEWIDE
 Contractor: #N/A
 Report to District: RMS
 Resident Engineer: #N/A

LABORATORY PREPARATION, COMPRESSIVE STRENGTH TESTING (T 22 / C39)

Specimen ID	Age	Unit	Break Date	Weight (lbs)	Diameter (in)	Area (in ²)	Load (lbf)	Strength (psi)	Average (psi)	Break Type
00086137	3	DAY	5/5/2025	8.33	4.02	12.69	33881	2669	5	
00086138	3	DAY	5/5/2025	8.33	4.02	12.69	35516	2798	5	
00086139	7	DAY	5/9/2025	8.33	4.02	12.69	62082	4891	4790	2
00086140	7	DAY	5/9/2025	8.33	4.02	12.69	60303	4751		2
00086141	7	DAY	5/9/2025	8.33	4.02	12.69	59862	4716		2
00086142	28	DAY	5/30/2025	8.33	4.02	12.69	80017	6304	6200	2
00086143	28	DAY	5/30/2025	8.33	4.02	12.69	79600	6271		2
00086144	28	DAY	5/30/2025	8.33	4.02	12.69	76338	6014		2
00086145	56	DAY	6/27/2025	8.33	4.02	12.69	85053	6701	6440	5
00086146	56	DAY	6/27/2025	8.33	4.02	12.69	81945	6456		2
00086147	56	DAY	6/27/2025	8.33	4.02	12.69	78111	6154		5

Break Type: Cone Cone & Split Columnar Shear Side Fracture Pointed

REMARKS

TESTING LABORATORY, TECHNICIAN(S), AND REVIEW

Laboratory: RMS	Location: HOPKINTON, MA
T 358 (Set 1): Jose Sanchez	Test Date: 5/9/2025
T 358 (Set 2): Jose Sanchez	Test Date: 5/30/2025
T 22 (Set 1): Frehiyot Tale	Test Date: 5/5/2025
T 22 (Set 2): Jose Sanchez	Test Date: 5/9/2025
T 22 (Set 3): Jose Sanchez	Test Date: 5/30/2025
T 22 (Set 4): Timothy Berard	Test Date: 6/27/2025
Reviewed By: Jose Sanchez	Review Date: 6/30/2025

SPECIFICATION LIMITS AND APPROVAL

Results are within specification limits: X Results are outside specification limits:

Approved By: _____
 Signature: _____
 Date: _____

*Results relate only to the items inspected or tested.
 **This report shall not be reproduced, except in full, without the prior written approval of the agency.

Critical Points During Construction

Mixing and Delivery

- **Hold-Back Water**
 - ≤10% of the approved **total water design**
- **Retempering**
 - Prior to the discharging of the first 1/3 of the load, no more than once

Handling and Placing Concrete

- **Transporting**
 - Chutes, pumps, buggies...
- **Depositing**
 - Operate come-alongs or flat headed shovels to move concrete in form; No toothed raking or dragging vibrators
- **Consolidation**
 - Vibration, Screeding, Bull Floating, etc.

Handling and Placing Concrete

Depositing

- Come-Alongs



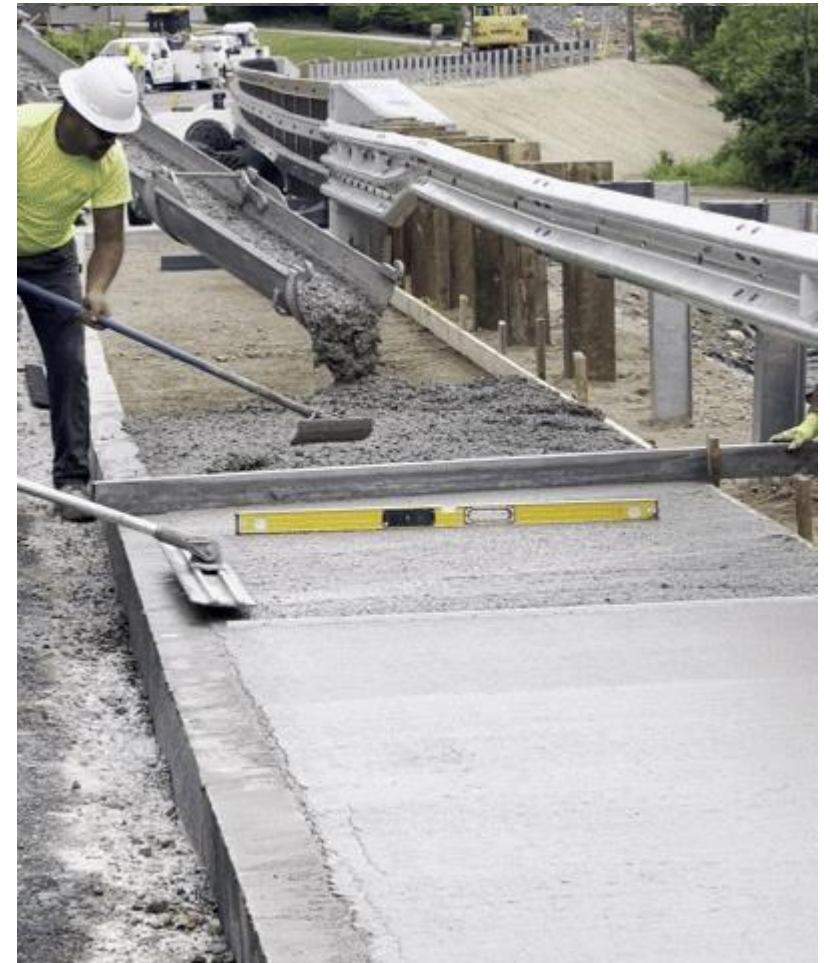
- Flat Headed Shovels



Critical Points During Construction

Finishing

- **Dormancy (Placed)**
 - Screeding and Bull Floating
- **At Initial Set (Stiffened), After Bleed**
 - Final Floating and Texturing
- **Cold Conditions (< 40°F)**
 - Heated Mixing Water, Type C Accelerating Admixtures...
- **Hot Conditions (Evaporation > Bleed)**
 - Liquid-Applied Evaporation Reducers, Fogging Nozzles, Sunshades, Canopies, Windbreakers, Ice...
 - Colloidal Silica Finishing Aids



Critical Points During Construction

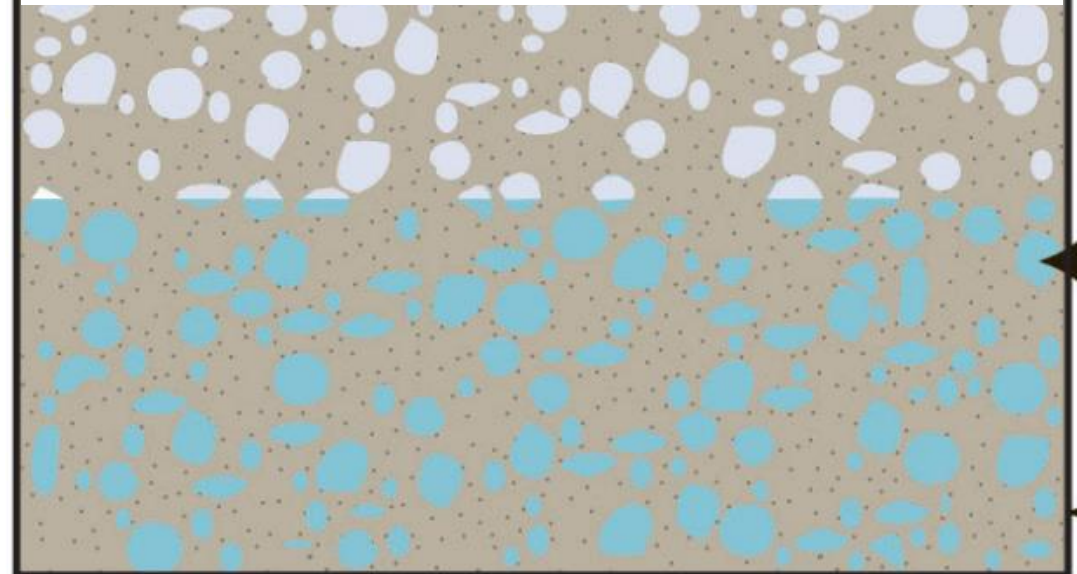
Evaporation Monitoring

- **Evaporation Rate of Surface Water**
 - $E = (T_c^{2.5} - r * T_a^{2.5})(1 + 0.4V) \times 10^{-6}$
- **Bleed Rate Determination Methods**
 - Assume: 0.45 w/cm = 0.15 lb/ft²/hr
 - Assume: 0.40 w/cm = 0.10 lb/ft²/hr
 - Mixes with < 0.40 w/cm, air > 7.5%, water reducers, or fine materials may have a lower bleeding rate
 - Test Panel: Placed concrete appears dry while the test panel accumulates water under transparent plastic = Evaporative Conditions
 - AASHTO T 158 Bleeding Test

Evaporative Conditions

Evaporation Rate > Bleed Rate

Plastic Shrinkage Cracking and Incomplete Hydration



Materials and Construction Best Practices

1. Right After Placing to Level Concrete

Screeding

- 2 x 4 in. Screed (Magnesium or Wood)



2. Right After Screeding to Push Aggregate Down

Bull Floating

- Magnesium Bull Float



2a. Hot Conditions (If Present)

Evaporation Reducing Materials

3. At Initial Set and Bleed Water Stops

Final Floating, Edging, Contraction Jointing

- Magnesium Hand Float



- Magnesium Darby Float



- Steel Edgers (Hand or Walking)



- Steel Groover (Hand or Walking, $D = 1/4T$)



4. Right After Final Floating, Edging, Joints

Texturing

- Soft-Bristled Broom that Produces 1/16 to 1/8 in. Deep Texture



4a. Hot Conditions (If Present)

Evaporation Reducing Materials

5. At Final Set

Curing Materials

Critical Points During Construction

Curing

- **At Final Set (Hardened)**
 - Curing Water Nozzles, Saturated Covers, Sheet Materials, Liquid Membrane-Forming Compounds (Curing or Curing and Sealing)...
 - Penetrating Colloidal Silica Compounds for curing and sealing
- **Cold Conditions (< 40°F)**
 - Insulating Materials, Heating, and Enclosures
- **Hot Conditions (Direct Sunlight)**
 - Sunshades, canopies, white polyethylene film, and white pigmented curing compounds



Critical Points During Construction

Protective Sealing

- **Usage and Adoption**
 - Non-existent due to application of Compounds for Curing and Sealing...but these aren't necessarily "true" sealers...
- **Surface Preparation**
 - Pressure Washing, Shotblasting, Abrasive Blasting, etc.
- **Surface Preparation Check**
 - Water absorption testing: ASTM F3191 or Penny Water Droplet Method
- **After Surface Preparation**
 - Penetrating Silanes, Siloxanes...
 - Colloidal Silica Compounds



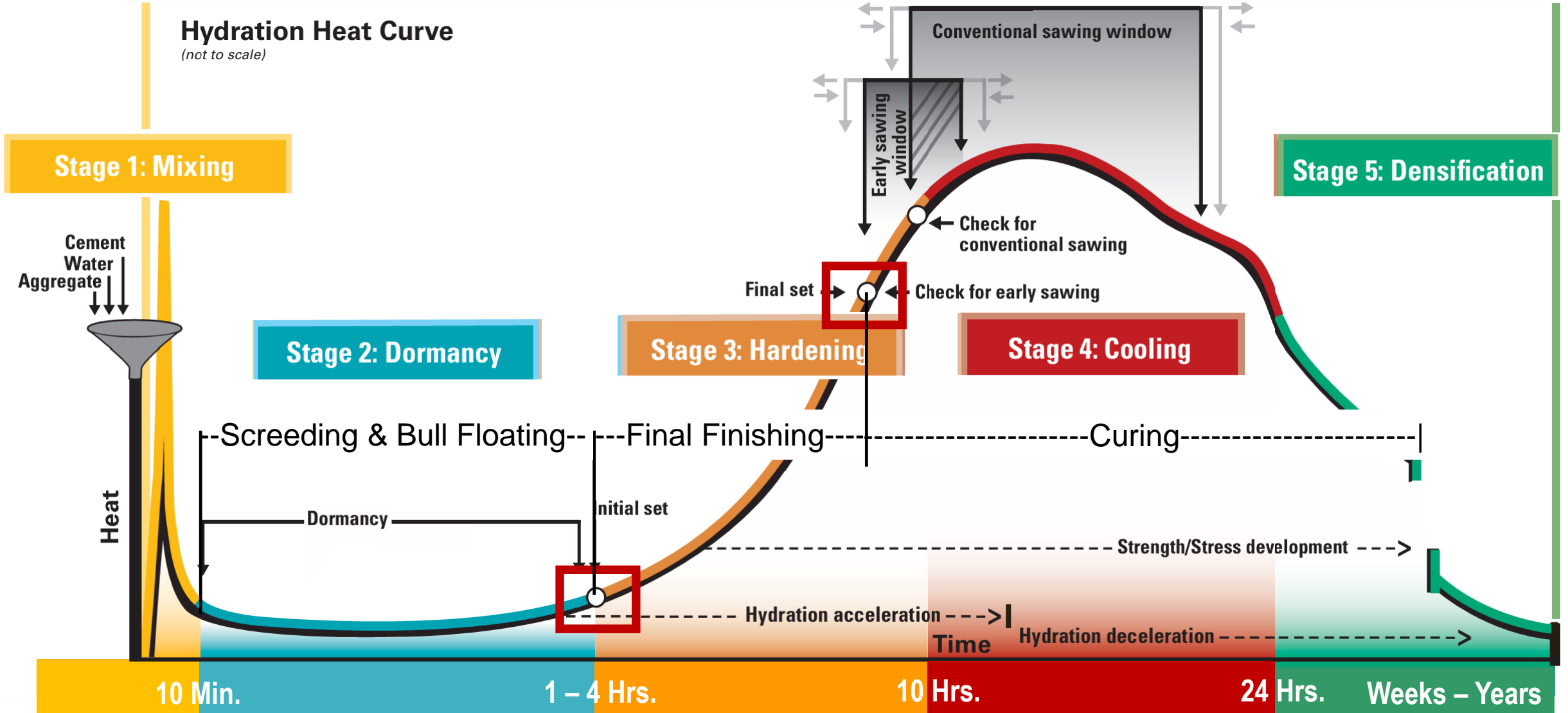
Critical Points During Construction

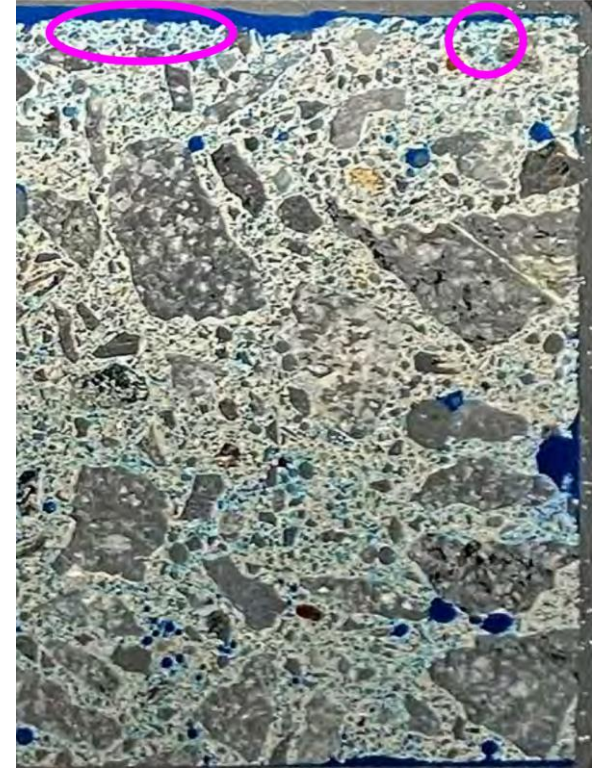
Contractor Quality Control (QC)

- **QC Operating Documents**
 - Quality System Manual and QC Plan
- **QC Laboratory**
 - Department Recognized Accreditation Program
- **QC Organization**
 - Frontline Personnel, QC Technicians, QC Manager
- **QC Sampling, Testing, and Inspection**
 - Random Sampling, Test Report Forms, Inspection Report Forms, and Non-Conformance Report Forms



Critical Points During Construction





Field Investigations and Petrographic Examination

Uncovering the mystery behind surface deterioration.



Spring 2026 – National Concrete Consortium

Current State of Things

- MassDOT, municipal, commercial, and residential exterior slab concrete exhibiting scaling
- All types of mix designs are exhibiting scaling...
 - 100% Type I/II cement
 - Type I/II cement and SCMs
 - 100% Type IL cement
 - Type IL cement and SCMs
- as well as NO scaling...



Current State of Things

Plant Distribution of Exterior Slab Concrete Mix Designs



Observations During Construction

Non-Conforming Activities

Mixing and Delivery

- Hold-Back Water

- >10% of the approved total water design

- Retempering

- After discharging of the first 1/3 of the load, multiple times

ORDERED: 50.00 DELIVERED: 10.00 SLUMP: 5.00

MAX WATER ALLOWED: 329.52 gl

WATER ADDED ALLOWED ON SITE: 34.01 gl

[AT PLANT] W/C RATIO: 0.41 WATER ADDED: 48.00 gl TOTAL AGGR WATER: 54.66 gl

BATCHED WATER: 240.85 gl

LOA

= 13.5% Hold-Back
(Spec. Limit ≤ 10%)

[ON SITE] W/C RATIO: 0.41 WATER ADDED: 0.00 gl

Observations During Construction

Non-Conforming Activities

Finishing

▪ Dormancy (Placed)

- No screeding and no bull floating. Use of roller tampers and steel fresnos.

▪ At Initial Set (Stiffened), After Bleed

- Final floating and texturing took place well before initial set and while bleed water was present. Blessed concrete. Worked in liquid-applied evaporation reducers and bleed water. Pushing of the broom. Did not remove water and paste from broom. Use of steel fresnos.

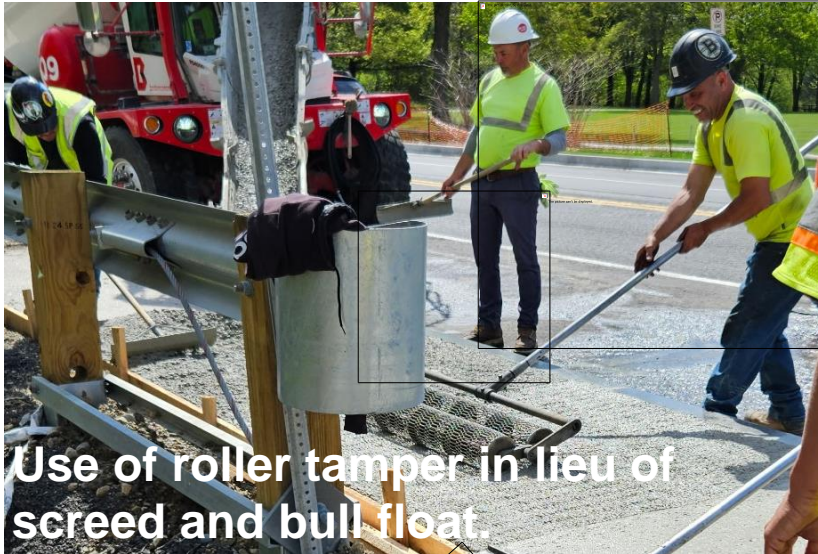
▪ Cold Conditions (< 40°F)

- No QC Plan and therefore no cold conditions planning or execution.

▪ Hot Conditions (Evaporation > Bleed)

- No QC Plan and therefore no hot conditions planning or execution. Worked in liquid-applied evaporation reducers and added water to the surface to compensate.

Observations During Construction



Use of roller tamper in lieu of screed and bull float.



Pushing broom. Use of walking floats.



Use of steel fresno in lieu of magnesium bull float.



Finishing in evaporation reducers. Use of inadequate bull float.

Observations During Construction



Overworking surface. Use of walking floats.



"Blessing" brush

Observations During Construction

Non-Conforming Activities

Curing

- **At Final Set (Hardened)**

- Application of Liquid membrane-forming compounds for curing and sealing took place well before final set, in direct conflict with the Manufacturer's TDS. Compounds often not applied per the required equipment, distance from surface, or the required coverage rate. In some cases, no attempt at curing at all...

- **Cold Conditions (< 40°F)**

- No QC Plan and therefore no cold conditions planning or execution. No Insulating Materials, Heating, and Enclosures.

- **Hot Conditions (Direct Sunlight)**

- No QC Plan and therefore no hot conditions planning or execution. No Sunshades, canopies, white polyethylene film, and white pigmented curing compounds.

Observations During Construction

Non-Conforming Activities

Contractor Quality Control (QC)

- Implementation

- Nonexistent or inconsistent. Frontline personnel require NRMCA Flatwork Certification but are in some cases not certified. Frontline personnel, while certified, do not follow the best practices that the course teaches. QC technicians are not on site to conduct inspection.



Observations Post Construction

No Scaling



Description

The surface remains intact with broomed finish still present.

Observations Post Construction

Minor Scaling



Description

Small, isolated scaling spots, with minimal surface flaking. Mortar cover intact. Depth of surface loss is minor (< 1/8 in.).

Observations Post Construction

Moderate Scaling



Description

Small scaling spots and surface flaking are more frequent but remain scattered, not clustered. Depth of surface loss is minor (< 1/8 in.).

Observations Post Construction

Major Scaling



Description

Scaling is clustered in small, isolated patches, with coarse aggregate potentially showing. Small scaling spots and surface flaking may also be scattered. Depth of surface loss is major (1/8 in. – 1/4 in.).

Observations Post Construction

Severe Scaling



Description

Scaling shows larger and more continuous clusters, often covering significant portions of the surface. Coarse aggregate is more commonly exposed. Depth of surface loss is major (1/8 in. – 1/4 in.).

Observations Post Construction

Very Severe Scaling



Description

Prevalent clustering with widespread surface deterioration. Mortar cover is largely gone in areas, with some coarse aggregate exposed and rough surface texture. Depth of surface loss is major (1/8 in. – 1/4 in.).

Observations Post Construction

Extreme Scaling



Description

Extensive surface loss with heavy clustering and broad aggregate exposure. Mortar cover is disintegrated and the surface is deeply deteriorated. Depth of surface loss is extreme ($>1/4$ in.).

Petrographic Examination

Coring of Contracts With Scaling

- Selected 14 Contracts
- Examine four (4) panels with scaling per contract
- Extract two (2) cores per panel
 - One (1) core with no scaling as a control with top paste layer intact
 - One (1) core with 1/3 area scaling and 2/3 area no scaling



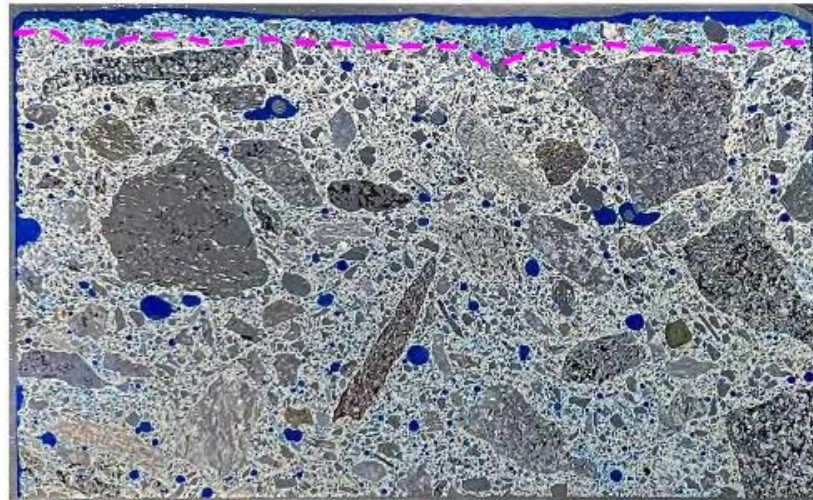
Petrographic Examination



Petrographic Examination

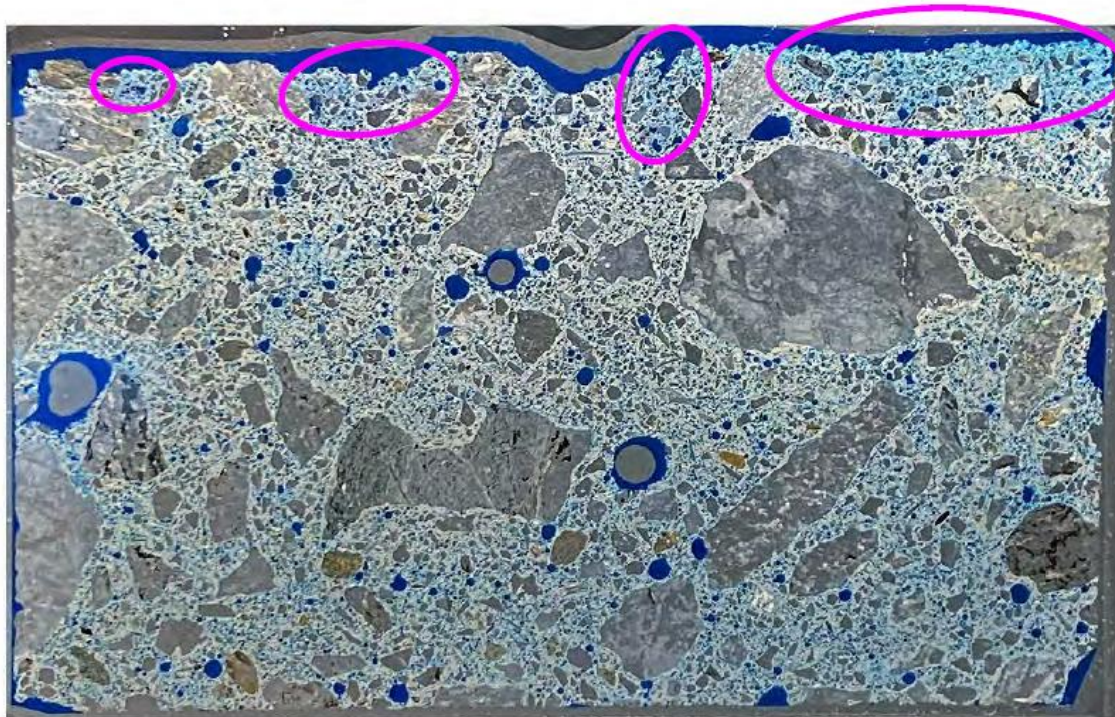
Laboratory Analysis

- ASTM C856 Petrography
- ASTM C457 Air Void System
- ASTM C1218 Chlorides



Project: 191814.06
Date: 9 July 2025

Petrographic Examination

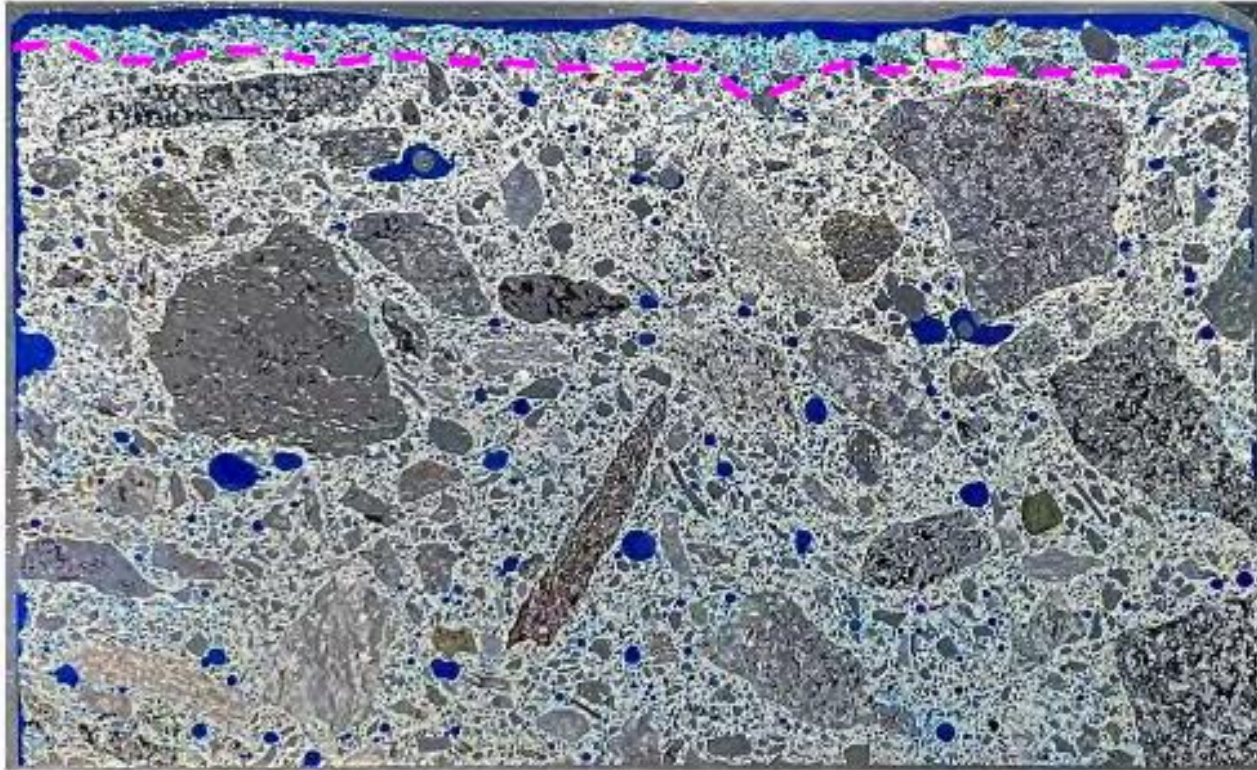


1 in

Description:

Overview of the Core 2B ultrathin section, oriented with the wearing surface at the top of the image. The sample was impregnated with blue-dyed epoxy, which highlights voids and areas with high levels of porosity in the cementitious paste (a thin layer of blue-dyed epoxy covers the wearing surface). The magenta ovals mark near-surface areas with strong saturation of blue, which are indicative of pockets of highly porous cementitious paste.

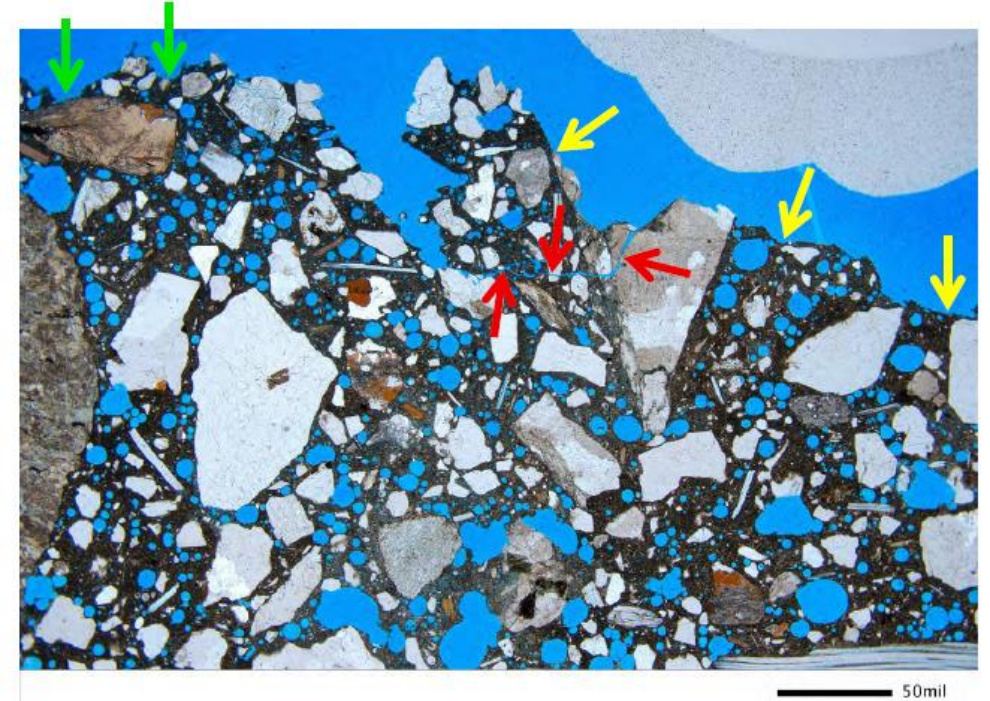
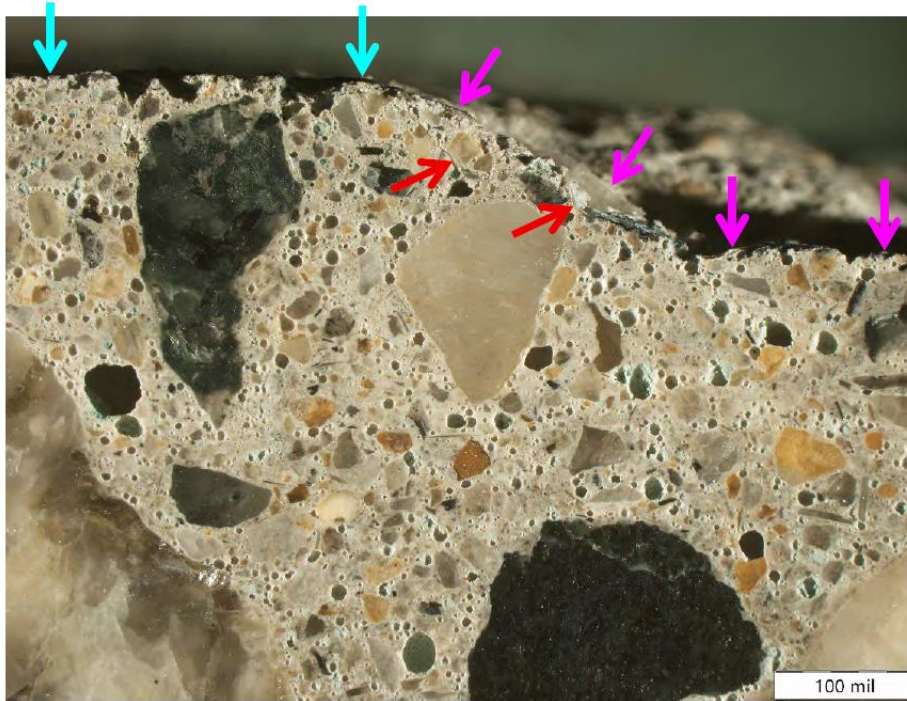
Petrographic Examination



Description:

2G ultrathin section, oriented with the wearing surface at the top of the image. The sample was impregnated with blue-dyed epoxy, which highlights voids and areas with high levels of porosity in the cementitious paste (a thin layer of blue-dyed epoxy covers the wearing surface). Note the strong saturation of blue in the area between the wearing surface and the magenta dashed line, which is indicative of a near-surface zone of highly porous cementitious paste.

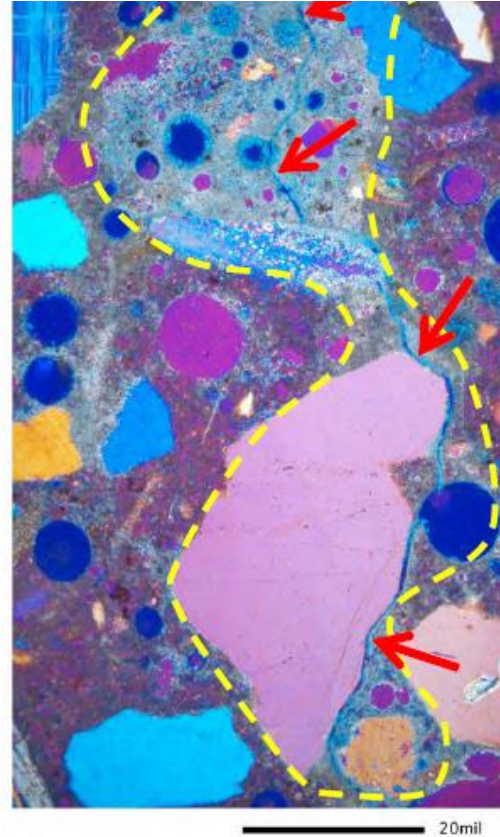
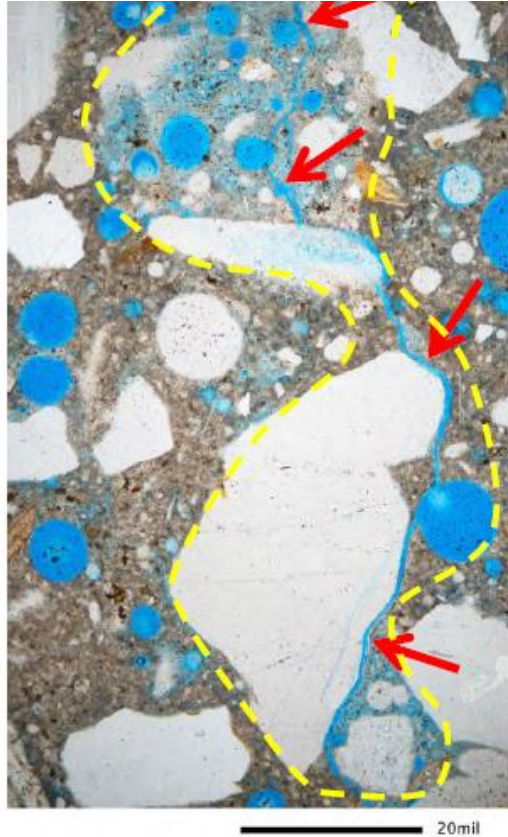
Petrographic Examination



Description:

Photomicrograph of the Core 2B polished section (left) and thin section (right), depicting a transition between an intact portion of the wearing surface and a scaled. The red arrows mark microcracks.

Petrographic Examination

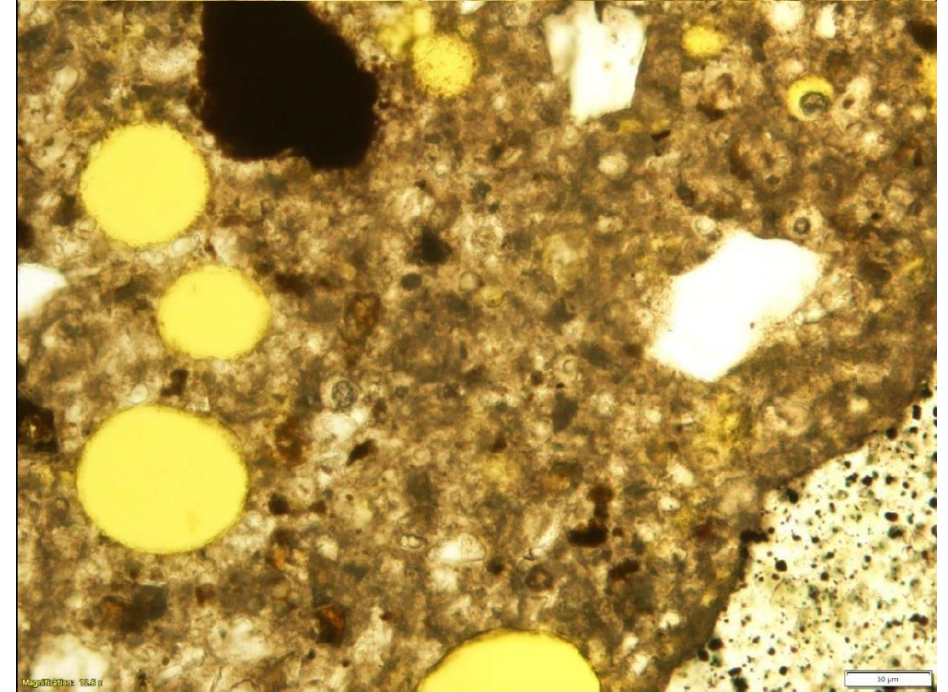
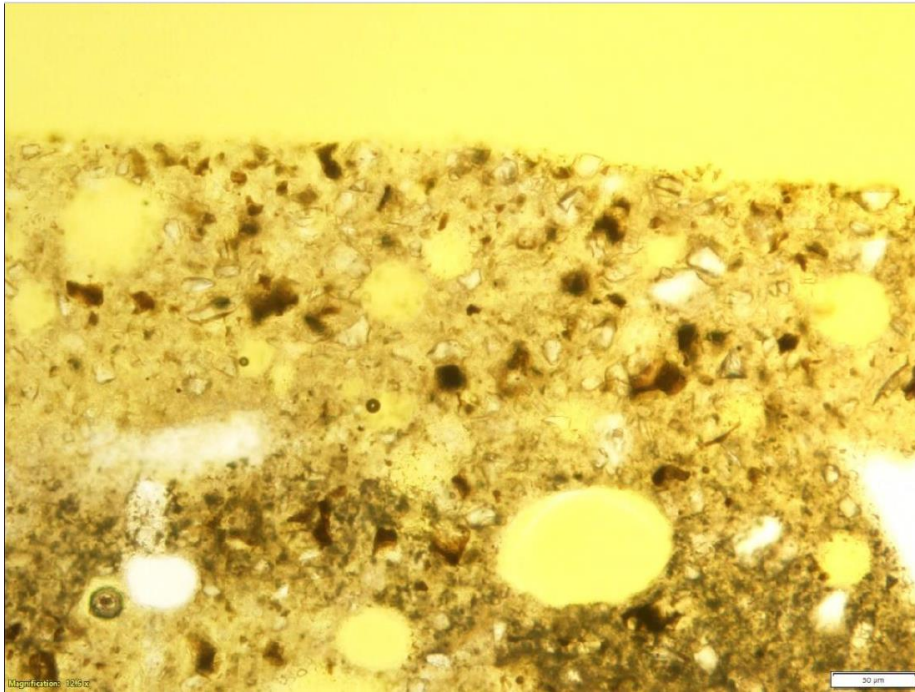


Description:

Photomicrograph mosaics of the Core 2G ultrathin section, depicting bleed-water channels (red arrows) extending through a near-surface zone of highly porous and carbonated cementitious paste (edge and lower boundary marked by yellow dashed lines). The wearing surface is at the top of the image.

(Plane polarized light on left; cross polarized light with full waveplate on right).

Petrographic Examination



Description:

Paste at the surface (Left picture) versus the bottom of core 25-1309A (Right picture), showing a denser, more hydrated paste deeper within the core. PPL, 200X magnification.

Findings

Top Layer (Surface Zone)

- The top layer consists of a high-porosity, low-cohesion cementitious paste
- The presence of a porosity gradient indicates the surface layer is materially different from the bulk concrete
- The shallow depth of deterioration confirms a surface-limited distress mechanism
- Increased porosity and reduced paste integrity make the surface susceptible to environmental exposure and mechanical breakdown

Findings

Underlying Concrete (Body)

- Paste is dense and uniform, typical of properly proportioned concrete
- No evidence of internal cracking or distress mechanisms
- Air content and spacing factor within acceptable limits (Spacing factor within 0.004 - 0.008 in.)
- Estimated w/cm ratio consistent with mix design (0.40 to 0.45)
- No indication of excessive water content in bulk matrix
- Chloride concentrations decreased with depth
- Carbonation is limited to near-surface region

Findings

Potential Causes of Scaling

- Improper finishing practices (inadequate screeding and bull floating, incorrect tools and timing, overworking surface)
- Addition of water at the surface
- Finishing in bleed water and/or evaporation reducers
- Uncontrolled evaporation (evaporation rate exceeding bleed rate)
- Insufficient curing



Emerging Chemical Admixtures and Topical Applications

A supplement for enhanced concrete durability?

Chemical Admixtures

Colloidal Silica (Type S-CSA)

- Stabilized amorphous silica particles ranging from 1 to 100 nm in diameter, polymerized from soluble silicate under alkaline conditions

Key Results from Research Project [1]

Test Method	Quality Characteristic	Result
AASHTO TP 142	Reduction in ASR from Control (%) [2]	82
AASHTO T 357	Reduction of Chloride Penetration from Control (%) [3]	33

[1] Type S-CSA: E5 Liquid FA / Internal Cure

[2] Control: Very Highly Reactive

[3] Control: High Chloride Penetration

Permeability Reducing (Type S-PRAN)

- Non-hydrostatic conditions; Increases water repellency; Reduces water absorption
- Long-chain fatty acid derivatives, soaps and oils, petroleum derivatives, silanes, and reactive or inert fine particle fillers (colloidal silica, silicates, silicious powders, clay, lime, bentonite, talc)

Permeability Reducing (Type S-PRAH)

- Hydrostatic conditions; Increases resistance to water penetration; Reduces corrosion; Blocks pores and capillaries
- Crystalline hydrophilic polymers that react with moisture and unhydrated cement compounds to form insoluble crystalline structures that block capillary pores

Colloidal Silica Finishing Aids

Specifications

- Currently no AASHTO or ASTM Standards
- Refer to Technical Data Sheet (TDS)

Qualified Construction Materials List

- Five (5) products listed
- [Colloidal Silica Finishing Aids* | Mass.gov](#)

Product Claims

“Lubricates the surface for faster, easier finishing. Extends workable time under adverse conditions such as high wind, heat, or low humidity. The amorphous silica reacts during the hydration of concrete to produce more cementitious material, translating into higher density and improved surface performance.” – Solomon Colors Day1

Product TDS Specifications

Test Method	Quality Characteristic	Min.	Max.
ASTM C672	Reduction in Scaling (%)	70	–
AASHTO T 161	Reduction of Mass Loss (%)	85	–
ASTM C779	Reduction in Abrasion (%)	35	–
AASHTO T 259 / T 260	Chloride Ion Penetration (%)	95	–

Note: Currently working on developing an AASHTO UP3 Technical Committee for Colloidal Silica Finishing Aids

Penetrating Colloidal Silica Compounds for Curing and Sealing

Specifications

- Currently no AASHTO or ASTM Standards
- Refer to Technical Data Sheet (TDS)

Qualified Construction Materials List

- One (1) product listed
- [Liquid Membrane-Forming \(M4.12.4\) & Colloidal Silica* Compounds | Mass.gov](#)

Product Claims

“Penetrates into the accessible capillary system, reacting with the available free alkali found within, and primarily forming CSH. Equal to or better than a 28-day moist cure. Applied at final set for curing. Can be applied to existing concrete as a penetrating sealing compound – Spray-Lock P3 Protect



Product TDS Specifications

Test Method	Quality Characteristic	Min.	Max.
ASTM C1556	Reduction in Chloride Diffusion (%)	30	–
AASHTO T 161	Reduction of Mass Loss (%)	40	–
ASTM E96	Reduction in Water Vapor Transmission of Materials (%)	70	–
EN 12390-8	Reduction in Depth of Penetration of Water Under Pressure (%)	70	–

Note: Currently working on developing an AASHTO UP3 Technical Committee for Penetrating Colloidal Silica Compounds for Curing and Sealing

Case Study – Overview

Project Scope

- Concrete sidewalk with premature deterioration due to scaling
- Remove and replace deteriorated panels
- Place same mix design formulation for new panels
- Place concrete in the same area and implement best construction practices
- Apply Solomon Colors Day1 colloidal silica finishing aids
- Apply ASTM C1315 liquid membrane-forming compounds for curing and sealing
- Apply Spray-Lock P3 penetrating colloidal silica compounds for curing and sealing



Placed: June 10, 2024

Photo: May 8, 2025

Case Study – Mix Design Formulation

Cementitious: 611 lb. **Type I/II: 65%** **Coal Ash (F): 15%** **Slag (120): 20%**

Mix Design Type: Exterior Slab Concrete (ESC) **Compressive Strength: 4,500 psi**

NMAS: 3/4 in. **Slump: 6 in.** **AC: 7.0%** **PC: 28.5%** **PC + AC / VC: 1.94**

Shilstone: Zone II (Optimum) **Tarantula Curve: Optimized, Close Conformance**

MIX DESIGN FORMULATION																																																										
MIX DESIGN ID NO.		MIX DESIGN TYPE	C (PSI)	NMAS (IN.)	S (IN.)	AC (%)	W/CM RATIO	PC (%)	AGGREGATE (LBS.)			CEMENTITIOUS; PACKAGED; FIBER (LBS.)					WATER (GAL.)		ADMIXTURES (FL. OZ.)						YIELD (CF)	UW (PCF)																																
DOT	PRODUCER								FINE	CA1	CA2	CA3	CEM	SCM1	SCM2	SCM3	PKG	FIBER	W _T	W _{ADMIX}	AD1	AD2	AD3	AD4			AD5	AD6																														
11	344034	ESC	4500	3/4	6	7.0	0.45	28.5	1117.0	725.0	1005.0	0.0	397.0	92.0	122.0	0.0	0.0	32.9	0.2	4.5	30.6	0.0	0.0	0.0	0.0	0.0	27.00	138.2																														
OPTIMIZED COMBINED AGGREGATE AND PASTE SYSTEM ANALYSIS																																																										
PERCENT BY MASS PASSING (%)												PERCENT RETAINED BY MASS (%) ON EACH SIEVE					SHILSTONE WORKABILITY-COARSENESS FACTOR			TCM	ECC	THICK																																				
2 IN.	1 1/2 IN.	1 IN.	3/4 IN.	1/2 IN.	3/8 IN.	#4	#8	#16	#30	#50	#100	#200	TARANTULA CURVE RESULT					CF	WF	ZONE		(LBS.)	(LBS.)	(FT.)																																		
100.0	100.0	100.0	97.5	75.6	66.2	45.6	38.0	32.1	22.0	10.0	2.6	0.8	OUTSIDE					54.5	39.3	II: 3/4 - 2 IN.		611.0	565.0	2.5																																		
Modified Andreason Distribution						Paste Content (%)		PC _A / VC _M Ratio		Tarantula Curve					Shilstone Chart			DEF/Thermal Crack Resistance																																								
																		<table border="1"> <tr> <th>T_{MAX} (°F)</th> <th>T_C (°F)</th> <th>T_A (°F)</th> </tr> <tr> <td>158.00</td> <td>70</td> <td>60</td> </tr> </table>			T _{MAX} (°F)	T _C (°F)	T _A (°F)	158.00	70	60																																
T _{MAX} (°F)	T _C (°F)	T _A (°F)																																																								
158.00	70	60																																																								
<table border="1"> <tr> <th>FM</th> <th>CA (%)</th> <th>q</th> <th>E (%)</th> <th>VC_A (%)</th> </tr> <tr> <td>4.86</td> <td>54.4</td> <td>0.41</td> <td>78.3</td> <td>28.4</td> </tr> </table>						FM	CA (%)	q	E (%)	VC _A (%)	4.86	54.4	0.41	78.3	28.4	<table border="1"> <tr> <th>PC (%)</th> </tr> <tr> <td>28.5</td> </tr> </table>		PC (%)	28.5	<table border="1"> <tr> <th>PC_A (%)</th> <th>VC_M (%)</th> <th>RATIO</th> </tr> <tr> <td>35.5</td> <td>18.3</td> <td>1.94</td> </tr> </table>		PC _A (%)	VC _M (%)	RATIO	35.5	18.3	1.94	OUTSIDE: 1/2 IN. #4 EXCESSIVE COARSE AGGREGATE RESULTING IN DECREASED WORKABILITY AND INCREASED SEGREGATION AND EDGE SLUMPING.					ZONE II (OPTIMUM FOR NMAS 3/4 IN. - 2 IN.): OPTIMIZED WORKABILITY FACTOR AND COARSENESS FACTOR			<table border="1"> <tr> <th>SPECIFIC APPLICATION</th> <th>THICK. (FT.)</th> </tr> <tr> <td>DS (SOIL)</td> <td>9.00</td> </tr> <tr> <td>DS (CASING)</td> <td>9.00</td> </tr> <tr> <td>DS (WATER)</td> <td>9.00</td> </tr> <tr> <td>ABUTMENT</td> <td>1.41</td> </tr> <tr> <td>PIER (ABOVE)</td> <td>1.77</td> </tr> <tr> <td>PIER CAP</td> <td>2.09</td> </tr> <tr> <td>PILE CAP</td> <td>3.18</td> </tr> <tr> <td>BRIDGE DECK</td> <td>9.00</td> </tr> <tr> <td>SLAB (GRADE)</td> <td>9.00</td> </tr> </table>			SPECIFIC APPLICATION	THICK. (FT.)	DS (SOIL)	9.00	DS (CASING)	9.00	DS (WATER)	9.00	ABUTMENT	1.41	PIER (ABOVE)	1.77	PIER CAP	2.09	PILE CAP	3.18	BRIDGE DECK	9.00	SLAB (GRADE)	9.00
FM	CA (%)	q	E (%)	VC _A (%)																																																						
4.86	54.4	0.41	78.3	28.4																																																						
PC (%)																																																										
28.5																																																										
PC _A (%)	VC _M (%)	RATIO																																																								
35.5	18.3	1.94																																																								
SPECIFIC APPLICATION	THICK. (FT.)																																																									
DS (SOIL)	9.00																																																									
DS (CASING)	9.00																																																									
DS (WATER)	9.00																																																									
ABUTMENT	1.41																																																									
PIER (ABOVE)	1.77																																																									
PIER CAP	2.09																																																									
PILE CAP	3.18																																																									
BRIDGE DECK	9.00																																																									
SLAB (GRADE)	9.00																																																									

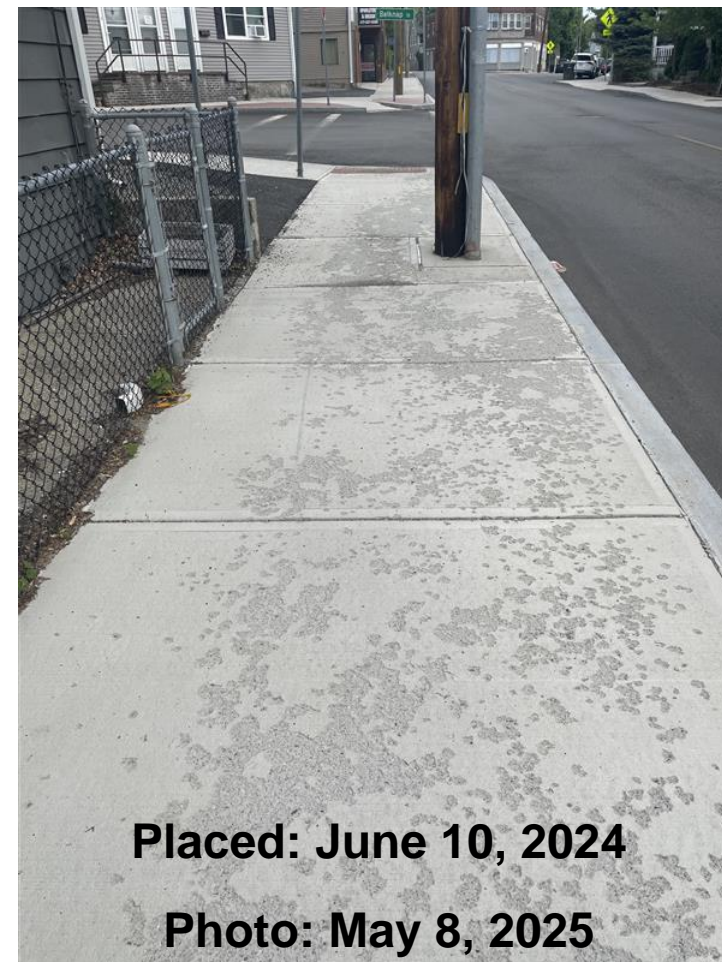


Case Study – Environmental Conditions

Placement 1 – Scaling Deterioration

$$E = (T_c^{2.5} - r * T_a^{2.5})(1 + 0.4V) \times 10^{-6}$$

Input	Measurement	0.45 w/cm Bleed Rate
T _c (°F)	77	
r (%)	50	
T _a (°F)	76	
V (mi/hr)	10 to 20	
E (lb/ft²/hr)	0.13 to 0.24	≤ 0.15
EVAPORATIVE CONDITIONS!		



Placed: June 10, 2024

Photo: May 8, 2025

Case Study – Environmental Conditions

Placement 2 – No Scaling Deterioration

$$E = (T_c^{2.5} - r * T_a^{2.5})(1 + 0.4V) \times 10^{-6}$$

Input	Measurement	0.45 w/cm Bleed Rate
T _c (°F)	70	
r (%)	75	
T _a (°F)	64	
V (mi/hr)	0 to 6	
E (lb/ft²/hr)	0.02 to 0.06	≤ 0.15
NO EVAPORATIVE CONDITIONS!		



Placed: September 16, 2025

Photo: March 27, 2026

Case Study – Apply Penetrating Colloidal Silica Compounds for Curing and Sealing



Placed: September 16, 2025



Case Study – Findings for Placement 1 (Scaled) vs. Placement 2 (Not Scaled)

Constants

- Mix design formulation (Type I/II, coal ash, and slag)
- Exposure conditions (freezing, thawing, and de-icing)

Variables

- Non-conforming vs. Best Construction Practices
- Evaporative Conditions vs. Ideal Conditions
- Incorrect vs. per TDS application of ASTM C1315 liquid-membrane forming compounds for curing and sealing
- Application of colloidal silica finishing aids and penetrating colloidal silica compounds for curing and sealing

Results

- Best materials and construction practices prevail
- Colloidal silica finishing aids did not damage the surface
- Panels applied with penetrating colloidal silica compounds for curing and sealing exhibited no scaling



Closing Remarks

There are no shortcuts for concrete...

Understand the importance of...

- Best Practices vs. Prohibited Practices
- Acceptable Tools vs. Prohibited Tools
- Contractor Quality Control
- **Initial Set:** Apply final floating and texturing
- **Evaporation Rate > Bleed Rate:** Apply evaporation reducing materials
- **Final Set:** Apply curing

Emerging materials are not silver bullets... They are supplements...

Producers, Contractors, and Agencies all share this burden to get this right!

Research, Specifications, and Webinars

- <https://www.mass.gov/lists/current-and-completed-research-projects>
- 2021 – Construction and Materials Best Practices for Concrete Sidewalks Subjected to Cold Conditions
- 2023 – Construction and Materials Best Practices for Concrete Sidewalks Subjected to Hot Conditions
- 2025 – Recycled Ground-Glass Pozzolan (RGGP) for Use in Cement Concrete and Comparison with Other Alternative Constituent Materials
- <https://www.mass.gov/lists/construction-specifications>
- <https://vimeo.com/1106214472>

Words of Encouragement!

Looks like you were right about the mix.
I know I was skeptical. But I am glad we stuck with the lower
cement factor with high range.
It gave us the strength needed and the lower paste factor.
I guess you are never too old to learn.

Norm



Thank You! Questions?

Richard Mulcahy, P.E.

Materials Research and Evaluation Engineer

MassDOT Research and Materials Section

Email: Richard.Mulcahy@dot.state.ma.us

Oussama Khouchani, Ph.D.

Research and Evaluation Technical Lead

MassDOT Research and Materials Section

Email: Oussama.A.Khouchani@dot.state.ma.us

